STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	BP13.R024	1	11

### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY MADISON

SITE DESCRIPTION BRIDGE NO. 560152 OVER SPILLCORN CREEK ON SR 1318 (BIG LAUREL ROAD)

### **CONTENTS**

SHEET NO. **DESCRIPTION** TITLE SHEET 2. 2A LEGEND (SOIL & ROCK) SUPPLEMENTAL LEGEND (GSI) 2B, 2C 3 SITE PLAN 4-II BORE LOGS, CORE LOGS, AND ROCK CORE PHOTOS PERSONNEL

CG2 EXPLORATION

S. N. PATTERSON, P.G.

INVESTIGATED BY CG2, PLLC

DRAWN BY S. N. PATTERSON, P.G.

CHECKED BY R. KRAL, P.E.

SUBMITTED BY CG2, PLLC

DATE \_\_*JUNE 2023* 

### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (99) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (INP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOL THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MADE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED. ON THE PROJECT, THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  I. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY MAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



**CHARLOTTE, NC 28227** (980) 339-8684



Robert E. Kral

06/02/2023

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**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

PROJECT REFERENCE NO.	SHEET NO.
BP13.R024	2

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS (PAGE 1 OF 2)

	(FAGE FOF 2)																		
					SOI	L DES	SCR)	PTI	ON				GRADATION						
BE PENE ACCORD IS	SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DISBB). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING; CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH											WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS							
4	AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,  VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6										ANGULARITY OF GUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:								
SOIL LEGEND AND AASHTO CLASSIFICATION											ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.								
GENERAL CLASS.	GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS											MINERAL OGICAL COMPOSITION  MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.							
GROUP	A-1	A-3		Α	1-2		A-4	A-5	A-6 A-7	A-1, A-2	A-4. A-5	Τ	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.						
CLASS.	A-1-a A-1-		A-2-4	A-2-5	A-2-6	A-2-7	3033331		A-7-5. A-7-6	A-3	A-6, A-7		COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LL < 31						
SYMBOL	000000000000000000000000000000000000000	o i						1.7.1					MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50						
% PASSING *10	50 MX									GRANULAR	SILT- CLAY	MUCK,	PERCENTAGE OF MATERIAL						
*40 *200	30 MX 50 M 15 MX 25 M	X 51 MN X 10 MX	35 MX	35 MX	35 MX	35 MX 3	36 MN	36 MN	36 MN 36 M	SOILS	SOILS	PEAT	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL						
MATERIAL PASSING *40													TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%						
LL PI	- 6 MX	– NP							40 MX 41 MN 11 MN 11 MN		S WITH LE OR	HIGHLY	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE						
GROUP INDEX	0	0	+	0	+		_	_	16 MX NO MX		erate NTS of	ORGANIC	GROUND WATER						
USUAL TYPES	STONE FRAG			SILTY O	R CLAY	EY	SIL	ΤY	CLAYEY		ANIC TTER	SOILS	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING						
OF MAJOR MATERIALS	GRAVEL, AND SAND	SAND		RAVEL			SOII		SOILS		· · <del>-</del> · ·		▼ STATIC WATER LEVEL AFTER 24 HOURS						
GEN, RATING AS SUBGRADE		EXCEL	LENT T	O GOOD				FAIR TO	POOR	FAIR TO	POOR	UNSUITABLE	PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA						
HJ JUDUNHUE		PI OF	A-7-5 9	SUBGROU	JP IS ≤	 ≤ LL - 30	; PI 0	F A-7-	SUBGROUP I	P00R 5 > LL - 30			SPRING OR SEEP						
		_	С	ONS	ISTE	NCY			ISENES				MISCELLANEOUS SYMBOLS						
PRIMARY	SOIL TYPE	:		ACTNES ISISTE		P			STANDARD RESISTENC LUE)		GE OF UNO PRESSIVE ! (TONS/F	STRENGTH	ROADWAY EMBANKMENT (RE)  ### SOIL DESCRIPTION  ### OF ROCK STRUCTURES						
GENERA GRANUL				RY LOO LOOSE				4 T(	10				SOIL SYMBOL  SOIL SYMBOL  STORY  SPT DAT TEST BORING  SLOPE INDICATOR INSTALLATION						
MATERI				DENSE	1 DENSE 10 TO 30 NSE 30 TO 50 DENSE > 50					N/A		ARTIFICIAL FILL (AF) OTHER							
GENERA	N I V		۷E	RY SO SOFT	FT			( 2 T			< 0.25		— INFERRED SOIL BOUNDARY — CORE BORING ● SOUNDING ROD						
SILT-C	LAY	MEDIUM STIFF STIFF			STIFF 4 TO 8					0.25 TO 0.5 0.5 TO 1.0 1 TO 2			INFERRED ROCK LINE  MW. MONITORING WELL  TEST BORING WITH CORE						
MATERI (COHES			VEF			15 TO 30 2 TI						TTTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER INSTALLATION - SPT N-VALUE							
				HARD TEX	KTUF	RE OF	R GF		SIZE		> 4		RECOMMENDATION SYMBOLS						
U.S. STD. SI				4		10	40		60 20				UNCLASSIFIED EXCAVATION - TOTAL UNCLASSIFIED EXCAVATION -						
OPENING (M	R (	OBBLE		GRAV	/EL	2.00	0.42 COARS SANE	Ε	0.25 0.0 FIN	E I	SILT	CLAY	SHALLOW UNCLASSIFIED EXCAVATION - EMBANKMENT OR BACKFILL						
(BLDR.		(COB.)		(GR		_	CSE. S	D.)	(F S	D.)	(SL.)	(CL.)	ABBREVIATIONS						
GRAIN MI SIZE IN		CO11	75 3	NICT:		2.0	חחר		0.25	0.05	0.00	5	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT						
	MOISTURE		E	1211		- LU B MOIS				TERMS	CTUBE OF	CCDIDTION	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\dot{\gamma}_{\rm d}$ - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC						
(AT	TERBERG I	.IMITS	)			SCRIPTI				IQUID; VERY			DMT - DILATOMETER TEST						
LL _	. <del> </del> LIQUI	D LIMI	т	_	(	SAT.)			FROM BELO	W THE GRO	DUND WATE	ER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK						
PLASTIC RANGE < (PI) PL	PI AS	TIC LII	міт		- WE	T - (W)				REQUIRES		0	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS " - MOISTURE CONTENT CBR - CALIFORNIA BEARING HI HIGHLY V - VERY RATIO						
0M					- MC	DIST - (	M)		SOLID; AT	OR NEAR O	PTIMUM MI	OISTURE	EQUIPMENT USED ON SUBJECT PROJECT						
SL													DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:  CME-45C CLAY BITS X AUTOMATIC MANUAL						
						(D) - Y				ADDITIONAL TIMUM MOIS		0	CME-55 G* CONTINUOUS FLIGHT AUGER CORE SIZE:						
PLASTICITY											X 8" HOLLOW AUGERS -B -B -H								
PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 VERY LOW								DEX (	<u> </u>	<u>D</u>	TUNGCARBIDE INSERTS								
SLIGHTLY PLASTIC											VANE SHEAR TEST CASING WY ADVANCER HAND TOOLS:								
	HLY PLAS					26 0	R MO				HIGH		PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER						
						CO	LOR						MOBIL F B-29   TRICONE TUNGCARB.   SOUNDING ROD						
										. YELLOW-B			X CORE BIT VANE SHEAR TEST						
	-D1, 1E11J	JJ011 F	.5 [10	<b>,</b> DF	3	. NEANEL	., . 10		JJLD 10	J_JC111DL P	LAINAINC								

BP13.R024

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### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS  $(PAGE \ 2 \ OF \ 2)$ 

ROCK DESCRIPTION HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.

ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES 3 100 BLOWS PER FOOT IF TESTED. FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.

FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.

COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC. CRYSTALLINE ROCK (CR) NON-CRYSTALLINE ROCK (NCR) COASTAL PLAIN SEDIMENTARY ROCK (CP) WEATHERING FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO SLIGHT 1 INCH, OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. (SLI.) MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH MODERATELY SEVERE (MOD, SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT SEVERE REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. (SEV.) IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VERY SEVERE (V SEV.) VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u> ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND COMPLETE SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS ROCK HARDNESS CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES VERY HARD SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED HARD TO DETACH HAND SPECIMEN. MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE

EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. CAN BE GROOVED OR GOLIGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFF OR PICK POINT. MEDILIM CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE HARD POINT OF A GEOLOGIST'S PICK. CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. SOFT VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY B FINGERNAIL.

FRACTURE SPACING BEDDING TERM TERM THICKNESS SPACING VERY WIDE MORE THAN 10 FEET 3 TO 10 FEET VERY THICKLY BEDDED THICKLY BEDDED 4 FEET 1.5 - 4 FEET 0.16 - 1.5 FEET WIDE THINLY BEDDED
VERY THINLY BEDDED
THICKLY LAMINATED MODERATELY CLOSE 1 TO 3 FEET CLOSE VERY CLOSE 0.03 - 0.16 FEET 0.008 - 0.03 FEET LESS THAN 0.16 FEET THINLY LAMINATED < 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. RUBBING WITH FINGER EREES NUMEROUS GRAINS. GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. MODERATELY INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE: INDURATED DIFFICULT TO BREAK WITH HAMMER. SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE: EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.

### TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

 $\underline{\text{DIP DIRECTION (DIP AZIMUTH)}}$  - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

 $\frac{\text{FLOAT}}{\text{PARENT}} - \text{ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.}$ 

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

 $\underline{\text{LEDGE}}$  - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVINING STRATIM

AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

<u>SAPROLITE (SAP.)</u> - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

<u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - I - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: (BL-4), N: 812844.934 E: 910712.457, ELEV 2009.8, -L- STA. 13+22.61 ELEVATION: 2009.8 FEET

FIAD = FILLED IMMEDIATELY AFTER DRILLING

ROADWAY DESIGN FILES PROVIDED TGS ENGINEERS ON DECEMBER 15, 2022

DATE: 8-15-14

BP13.R024 **2B** 

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND GEOLOGICAL STRENGTH INDEX (GSL) TARLES

SUPPLEMENTAL LEGEND, GEOLOGIES FROM AASHTO LRFD BRIDGE I	DES.	IGN SPE	CIFICATIO	ONS (PAC	I) TABLE GE 1 OF	S 2)
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)  From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS	VERY GOOD  Very rough, fresh unweathered surfaces	6000 Rough, slightly weathered, iron stained Starfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments	<b>VERY POOR</b> Slickensided, highly weathered surfaces with soft clay coatings or fillings
STRUCTURE		DEC	REASING SI	JKFALE UUA	HLIIY ==	
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities	CES	90			N/A	N/A
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets	F ROCK PIECE!		70 60			
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	RLOCKING OF		5	0		
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	INTE			40	30	
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	DECREASING				20	
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	- II -	N/A	N/A			10

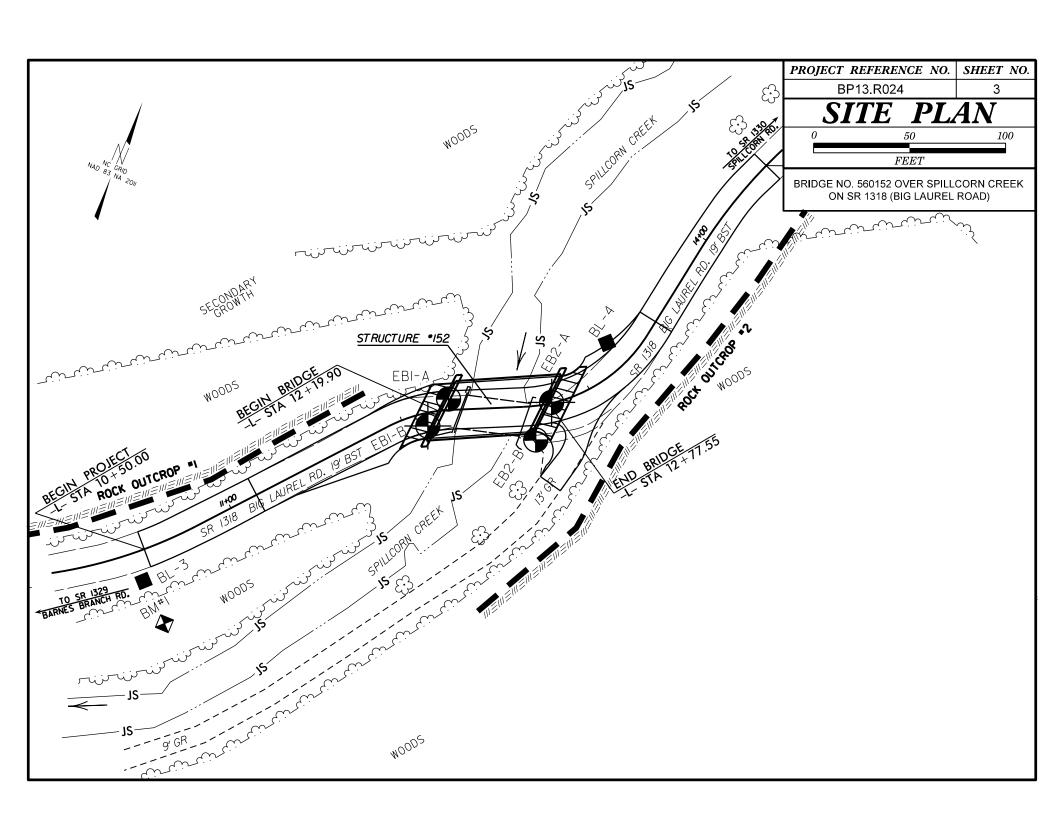
PROJECT REFERENCE NO.	SHEET NO.
BP13.R024	2C

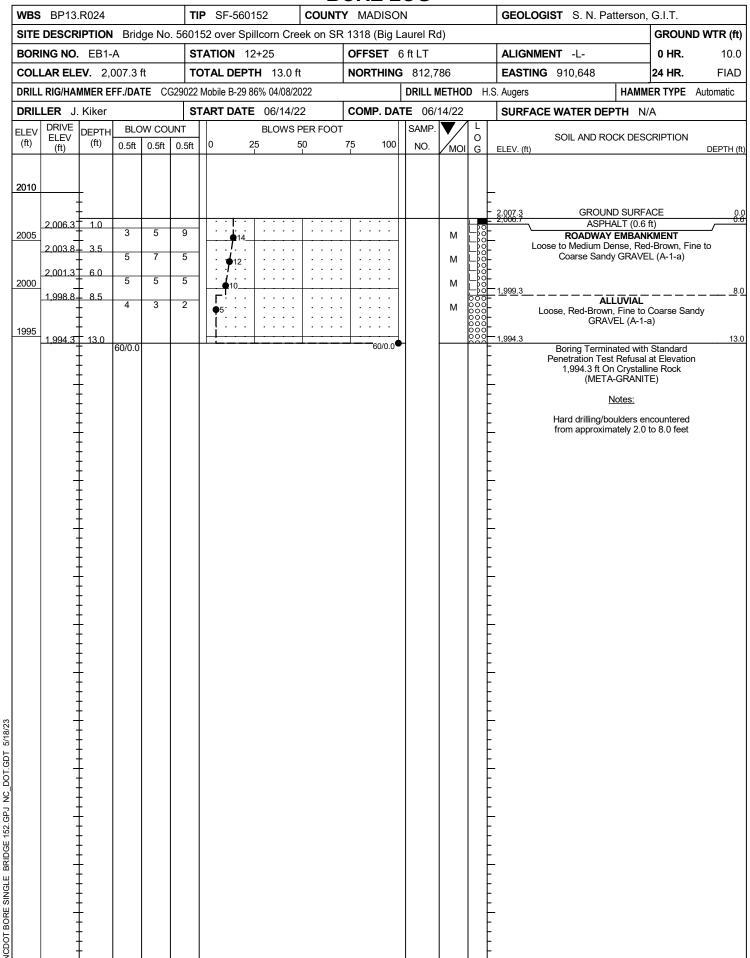
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

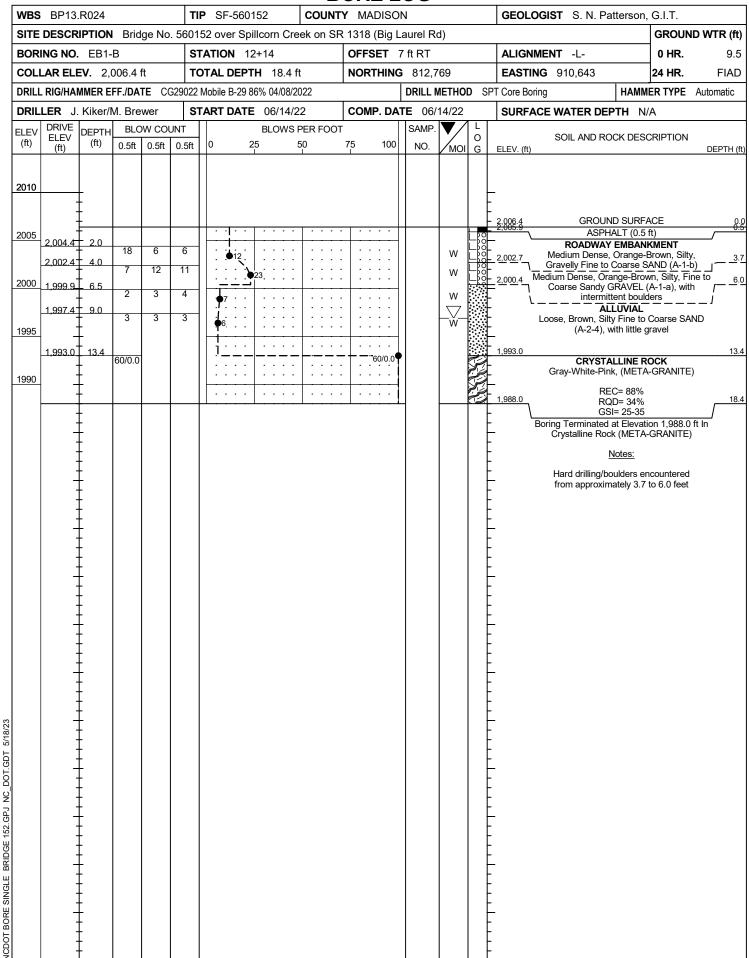
# SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS (PAGE 2 OF 2)

FROM $AASHTO$ $LRFD$ $BRIDGE$ $DESIGN$ AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically De					•
GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000)					
From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.  COMPOSITION AND STRUCTURE	Φ <del>-</del> <u>-</u>	G00D - Rough, slightly weathered surfaces	FAIR - Smooth, moderately weathered and altered surfaces	POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments	VERY POOR - Very smooth, slicken- sided or highly weathered surfaces with soft clay coatings or fillings
A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass, in shallow tunnels or slopes these bedding planes may cause structurally controlled instability.	70 60	A			
B. Sand- stone with stone and siltstone in similar amounts  C. Sand- solve and solve a		50 B 40	C I	E	
C.D.E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.			30	F/ 20	
G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers  The first of clay. Thin layers of sandstone are transformed into small rock pieces.			\$		10
─────────────────────────────────────					DATE: 8-19-16







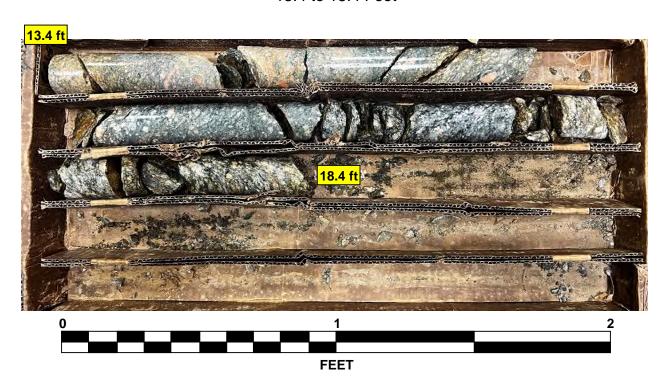
					_			-			(E L						
	BP13					SF-56					ADISO		(	GEOLOGIST S. N	I. Patterson,		
				lge No. 5			-	Creek	on SR	_		aurel Rd)				GROUN	ID WTR (ft
BORI	NG NO	. EB1	-B		STA	ΓΙΟΝ	12+14			OF	SET	7 ft RT		ALIGNMENT -L-		0 HR.	9.5
COLL	AR ELI	<b>EV.</b> 2,	006.4	ft	TOT	AL DE	<b>PTH</b> 18	.4 ft		NO	RTHING	812,769	E	<b>EASTING</b> 910,643	3	24 HR.	FIAD
DRILL	RIG/HA	MMER E	FF./DA	TE CG29	022 Mo	bile B-29	9 86% 04/0	08/2022				DRILL METHOD	SPT C	Core Boring	HAMM	ER TYPE	Automatic
DRILI	LER J	. Kiker/	M. Bre	ewer	STAI	RT DA	<b>FE</b> 06/1	4/22		СО	MP. DA	TE 06/14/22	5	SURFACE WATER	DEPTH N	/A	
CORE	SIZE	NQ			тот	AL RUI	<b>1</b> 5.0 ft						'				
ELEV	RUN	DEPTH	RUN	DRILL	REC.	JN RQD	SAMP.	STF REC.	RATA	L			DEC	DODIDTION AND DEA	44 DICO		
(ft)	ELEV (ft)	(ft)	(ft)	RATE (Min/ft)	(ft) %	(ft) %	NO.	(ft) %	(ft) %	O G	ELEV. (	ft)	DES	SCRIPTION AND REM	MARKS		DEPTH (
1993													В	Begin Coring @ 13	.4 ft		
	1,993.0	13.4 1	5.0	N=60/0.0 11:35/1.0 3:13/1.0 3:11/1.0 2:26/1.0 3:12/1.0	(4.4) 88%	(1.7) 34%		(4.4) 88%	(1.7) 34%		1,993.0	Verv Slightly to Me	/loderati	CRYSTALLINE RO ly Weathered, Hard to		rav-White-	13 Pink.
1990	-	Ŧ		3:13/1.0 3:11/1.0 2:26/1.0							_	(MEŤA-ĞRÁNITE)	), with \	Very Close to Close F to Thin Bedding	racture Spacin	ng and Very	/ Thin
-	1,988.0	18.4		3:12/1.0							1,988.0	٦		GSI= 25-35			18.
		Ŧ									•	Boring Ter	erminate	ed at Elevation 1,988.		ne Rock	
	-	‡									-			(META-GRANITE	=)		
		‡									•			Notes:			
	-	‡									-			d drilling/boulders end n approximately 3.7 to			
		‡											1101	Trapproximatory 6.7 to	0.0 1000		
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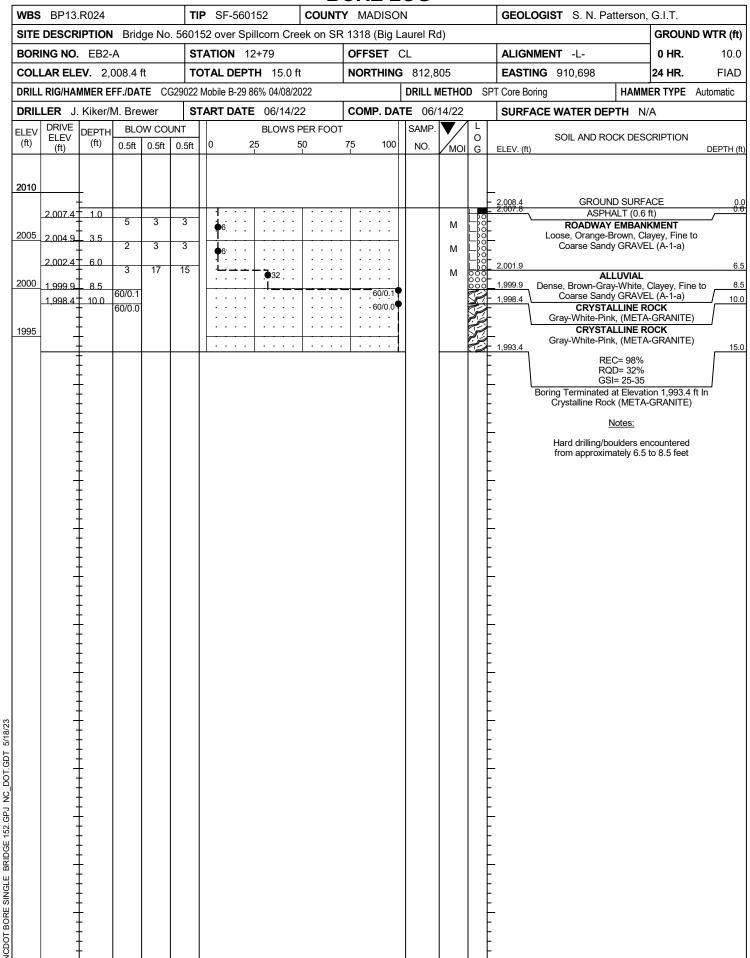


# Bridge No. 560152 over Spillcorn Creek on SR 1318 (Big Laurel Rd), Madison County, NC

# **Rock Core Photographs**

**Boring: EB1-B** 13.4 to 18.4 Feet





										<u> </u>	\ <u>_</u> _	.UG					
WBS	BP13	.R024			TIP	SF-56	0152	С	OUNT	<b>Y</b> N	IADISC	N		GEOLOGIST S. N. F	atterson,	G.I.T.	
SITE	DESCR	RIPTION	l Brid	ge No. 50	60152	over S	Spillcorn	Creek	on SR	131	8 (Big I	_aurel Rd)				GROUN	D WTR (ft)
BORII	NG NO	. EB2-	-A		STA	ΓΙΟΝ	12+79			OF	FSET	CL		ALIGNMENT -L-		0 HR.	10.0
COLL	AR EL	<b>EV</b> . 2,	008.4 1	ft	тот	AL DE	<b>PTH</b> 15	.0 ft		NO	RTHING	<b>3</b> 812,805		<b>EASTING</b> 910,698		24 HR.	FIAD
DRILL	RIG/HA	MMER E	FF./DA	TE CG29	022 Mo	bile B-29	9 86% 04/0	08/2022				DRILL METH	IOD SPT	Γ Core Boring	НАММ	ER TYPE	Automatic
		. Kiker/l					<b>TE</b> 06/1			СО	MP. DA	TE 06/14/22	2	SURFACE WATER DE	EPTH N/	'A	
	SIZE				TOTA	AL DIE	<b>v</b> 5.0 ft										
LEV	RUN	DEPTH	RUN	DRILL	RI	JN RQD (ft)	SAMP.		ATA RQD	L							
(ft)	ELEV (ft)	(ft)	(ft)	RATE (Min/ft)	REC. (ft) %	(ft) %	NO.	REC. (ft) %	(ft)	OG	ELEV. (	Tft)	Di	ESCRIPTION AND REMAR	RKS		DEPTH (ft
998.4	( )			, ,	//	70		70	70			,		Begin Coring @ 10.0 f	÷t		<i>32.</i> (
	1,998.4	10.0	5.0	N=60/0.0 4:55/1.0	(4.9) 98%	(1.6) 32%		(4.9) 98%	(1.6) 32%		1,998.4			CRYSTALLINE ROCK ratly Weathered, Hard to Ve		rov Mhito [	10.0
995	-	‡		4:55/1.0 3:34/1.0 2:58/1.0 3:14/1.0 3:03/1.0	90 /0	32 /0		3070	32 /0		-	(META-GRAN	NITE), with	h Very Close to Close Fract	ure Spacin	g and Very	Thin
-	1,993.4	15.0		3:14/1.0							1,993.4	٦		to Thin Bedding			15.0
		<u> </u>									<b>-</b> -	Borin	ng Termina	GSI= 25-35 ated at Elevation 1,993.4 ft	In Crystallir	ne Rock	
	-	†									<del>-</del>		•	(META-GRANITE)	•		
		ł									-			Notes:			
		ŧ									-			lard drilling/boulders encour rom approximately 6.5 to 8.			
	-	ŧ									-		11	от аррголіпасыў 0.0 го о.	J 1051		
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# Bridge No. 560152 over Spillcorn Creek on SR 1318 (Big Laurel Rd), Madison County, NC

## **Rock Core Photographs**

**Boring: EB2-A** 10.0 to 15.0 Feet



